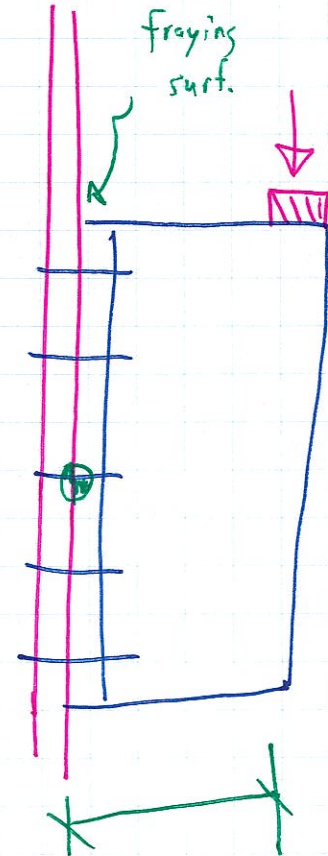


1) Load in-plane of fraying surface  
(Shear Only)



2) Load not in-plane of fraying surf.  
(bolts in shear & tension)

### ECCENTRIC LOAD - SHEAR ONLY

1) Elastic Method/Analysis - Simplified Method, but inaccurately conservative

SUMMARY

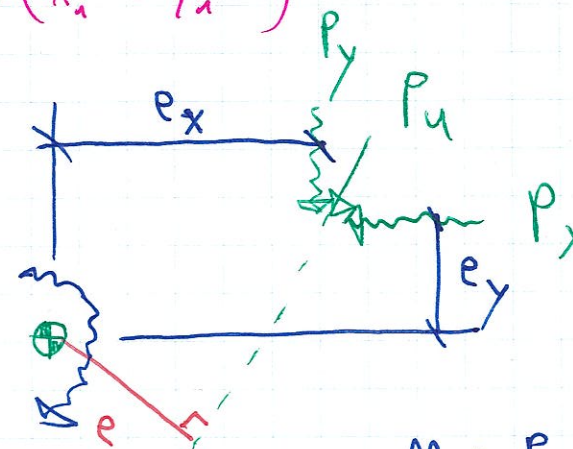
Force due to direct shear

$$P_{cx} = \frac{P_x}{n} ; P_{cy} = \frac{P_y}{n}$$

Force due to moment

$$P_{mx_i} = \frac{M y_i}{\sum (x_i^2 + y_i^2)} ; P_{my_i} = \frac{M x_i}{\sum (x_i^2 + y_i^2)}$$

where:  $M = \pm P_x e_y \pm P_y e_x$

 $P_x$  = applied force in x $P_y$  = " " " y $x_i$  = horz. dist. from CG to bolt i $y_i$  = " " " " " i

$$M = P_x e_y - P_y e_x = P_u e$$

Resultant Force

$$P_i = \sqrt{(P_{cx} \pm P_m x_i)^2 + (P_{cy} \pm P_m y_i)^2}$$

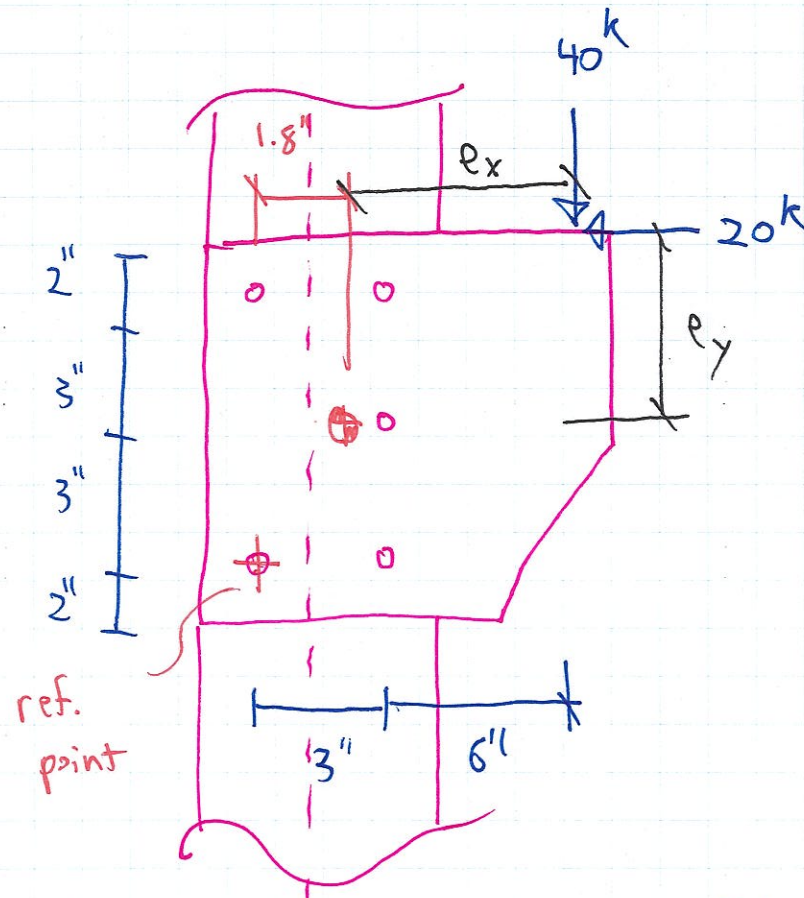
## 2) Method of Instantaneous Centers / Ult. Strength Analysis

- More accurate, but iterative & complex
- AISC part 7 uses this method - Provide a coef. "c" which is the ratio of design strength of bolt group to the design strength of a single bolt

$$c = \frac{P_u}{\phi r_n} = f(\text{bolt spacing, eccentricity, \# of bolts, arrangement of bolts, \& angle of load, etc.})$$

EXAMPLE

GIVEN: A plate bolted to a column flange.  $\downarrow$



Determining

CG of bolts

$$\bar{y} = \frac{(1 \text{ bolt})(3'') + (2 \text{ bolts})(6'')}{(5 \text{ bolts})} = 3.0''$$

$$\bar{x} = \frac{(3 \text{ bolts})(3'')}{(5 \text{ bolts})} = 1.8''$$

REQ'D: a) Max bolt shear force (elastic method)  
b) What size A325 bolts are required?

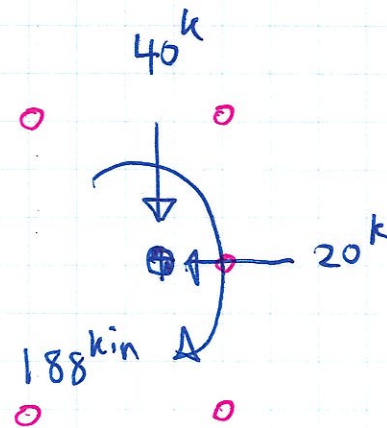
SOLN: a)

$$e_x = 9'' - 1.8'' = 7.2''$$

$$e_y = 3'' + 2'' = 5''$$

MOMENT:  $\overset{+}{M} = \pm P_x e_y \pm P_y e_x$

$$= +40^k(7.2'') - 20^k(5'') = 188^k\text{-in}$$



DIRECT SHEAR:

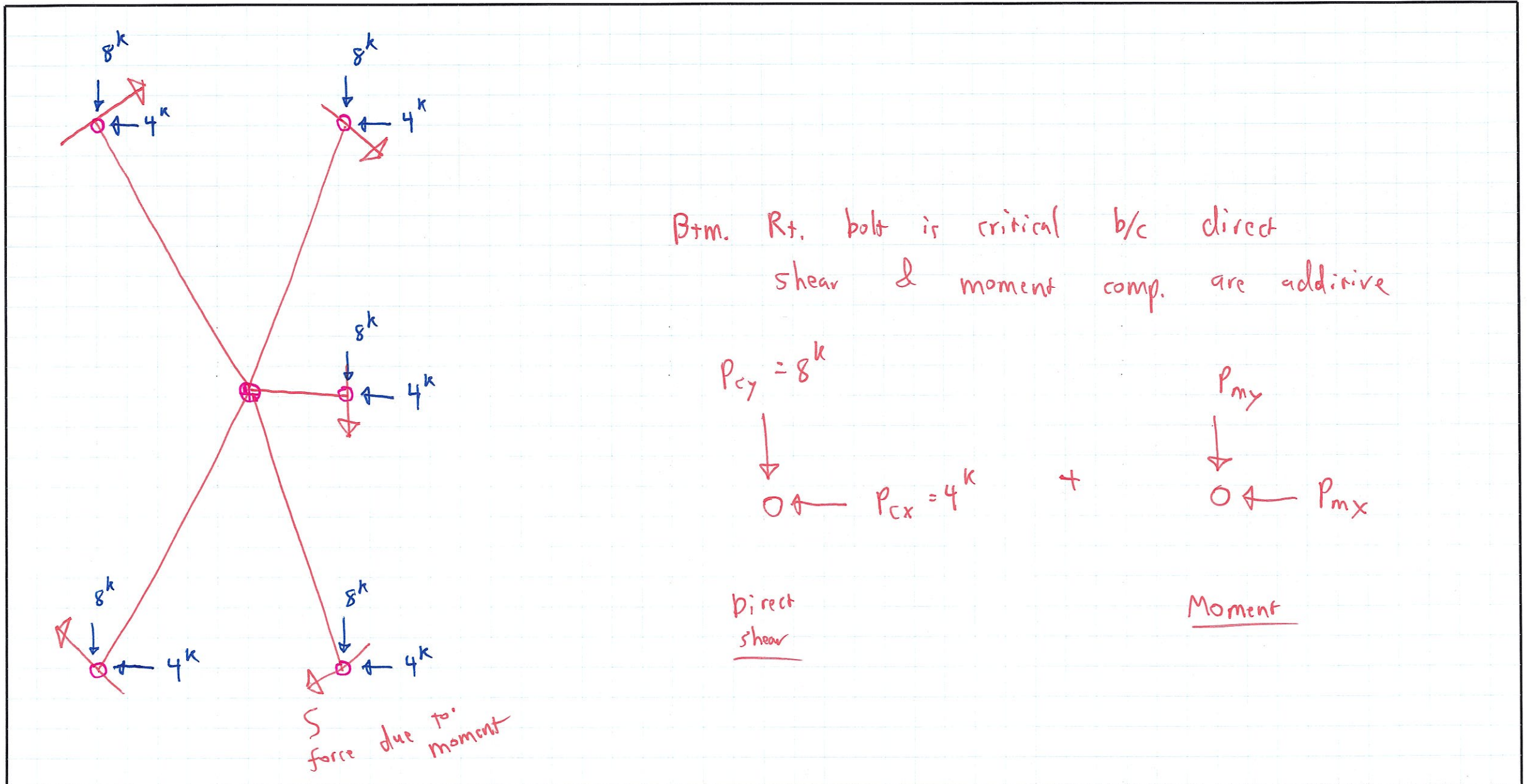
$$P_{cx} = \frac{20^k}{5 \text{ bolts}} = 4^k/\text{bolt} \quad \leftarrow$$

$$P_{cy} = \frac{40^k}{5 \text{ bolts}} = 8^k/\text{bolt} \quad \downarrow$$

SHEAR DUE TO MOMENT:

$$\sum (x_i^2 + y_i^2) = (2 \text{ bolts})(1.8'')^2 + (3 \text{ bolts})(1.2'')^2 + \underbrace{(2 \text{ bolts})(3'')^2 + (2 \text{ bolts})(3'')^2}_Y$$

$$= 46.8 \text{ in}^2$$



$$P_{mx} = \frac{M_y}{\sum (x_i^2 + y_i^2)} = \frac{188 \text{ k-in} (3")}{46.8 \text{ in}^2} = 12.05 \text{ k} \quad \leftarrow$$

$$P_{my} = \frac{M_x}{\sum (x_i^2 + y_i^2)} = \frac{188 \text{ k-in} (1.2")}{46.8 \text{ in}^2} = 4.82 \text{ k} \quad \downarrow$$

Total Shear

$$\begin{array}{c} \leftarrow + \\ \sum P_x \end{array} = P_{cx} + P_{mx} = 16.05 \text{ k} \quad \leftarrow$$

$$\begin{array}{c} + \downarrow \\ \sum P_y \end{array} = P_{cy} + P_{my} = 12.82 \text{ k} \quad \downarrow$$

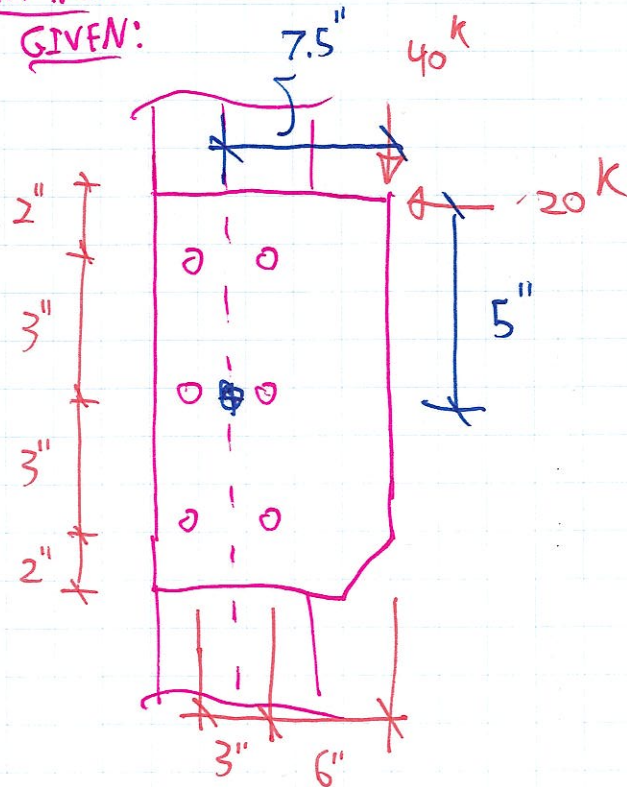
$$P = \sqrt{16.05^2 + 12.82^2} = \underline{\underline{20.5 \text{ k}}}$$

b) AISC Table 7-1, p7-22

$$\frac{7}{8} \text{'' } \phi, A325N, S, STD: \phi r_n = 24.3 \text{ k/bolt} > P = 20.5 \text{ k/bolt} \quad \checkmark$$

EXAMPLE

GIVEN:



$R = P_u = \sqrt{20^2 + 40^2} = 44.7 \text{ k}$

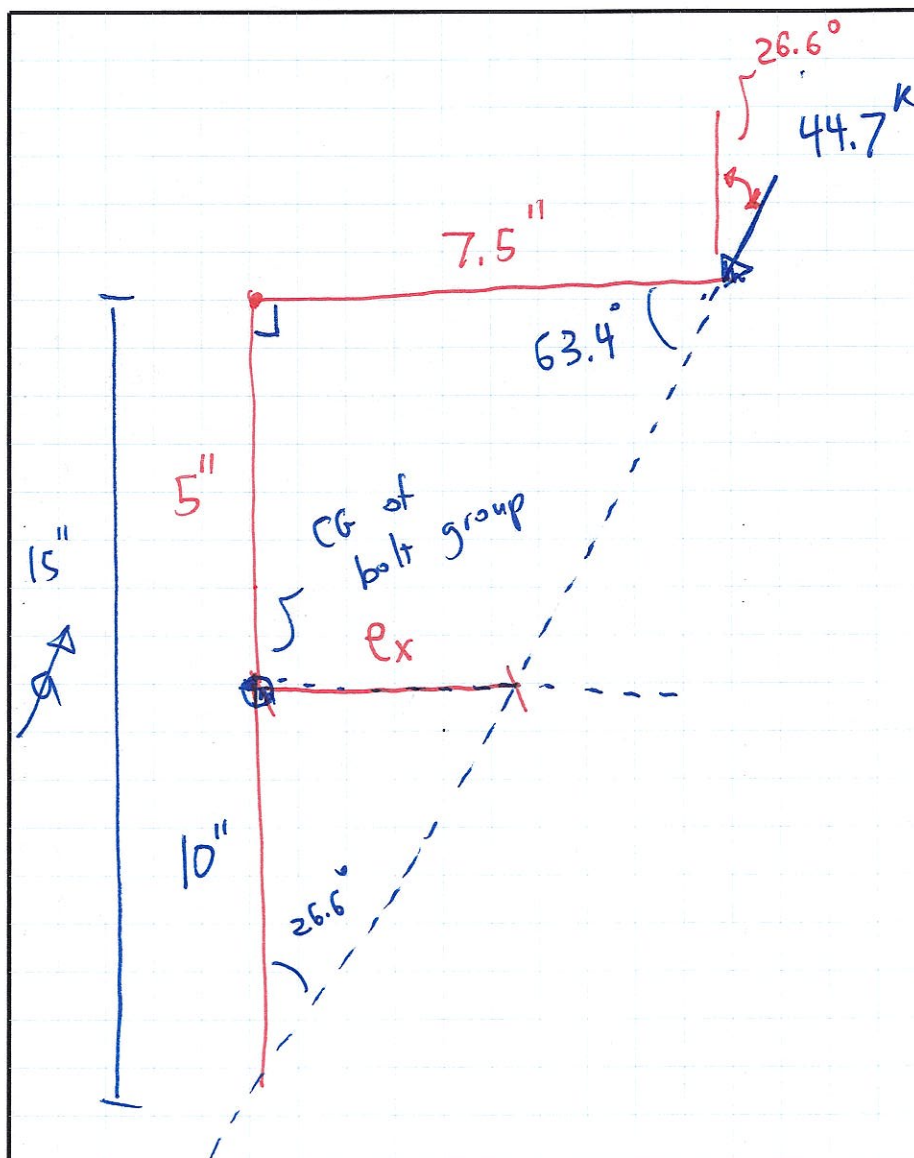
$\phi = \tan^{-1}\left(\frac{40}{20}\right) = 63.43^\circ$

$\theta = 90 - \phi = 26.6^\circ$

REQ'D: Determine the size of A325 bolts req'd for the bearing type connection:

- a) threads included
- b) threads excluded
- c) slip-critical

Use Ult. Strength Analysis



$$\frac{7.5''}{\sin(26.6^\circ)} = \frac{a}{\sin(63.4^\circ)} \Rightarrow a = 15''$$

(law of sines)

$$\frac{e_x}{10''} = \frac{7.5''}{15''} \Rightarrow e_x = 5''$$

(similar triangles)

$$S = 3''; e_x = 5''; n = 3 \rightarrow$$

$$C = 2.66 @ 15^\circ$$
$$C = 2.85 @ 30^\circ$$

} Table 7-7, p7-38 & 39

$$C = 2.66 + \frac{(2.85 - 2.66)}{(30 - 15)} (26.6 - 15) = 2.81$$

$$C = \frac{P_u}{\phi r_n} \Rightarrow (\phi r_n)_{\text{req'd}} = \frac{P_u}{C} = \frac{44.7^k}{2.81} = 15.93^k/\text{bolt}$$

- a)  $\frac{3}{4}'' \phi, A325N, S, STD$   $\phi r_n = 17.9^k/\text{bolt} > 15.9^k/\text{bolt}$  ✓
- b)  $\frac{3}{4}'' \phi, A325X, S, STD$   $\phi r_n = 17.9^k/\text{bolt}$  ✓
- c)  $1'' \phi, A325SC, S, STD$   $\phi r_n = 17.3^k/\text{bolt} > 15.9^k/\text{bolt}$  ✓
- } Table 7-1, p7-22
- Table 7-3, p7-24